

PROHLÁŠENÍ O VLASTNOSTECH
DoP Nr. MKT-312 - cz

1. Jedinečný identifikační kód typu výrobku: **MKT Chemická kotva VMZ a VMZ-IG**
2. Typ, série nebo sériové číslo nebo jakýkoli jiný prvek umožňující identifikaci stavebních výrobků podle čl. 11 odst. 4:
ETA-04/0092, Annex A3 & A5
Číslo šarže: viz obal výrobku
3. Zamýšlené použití nebo zamýšlená použití stavebního výrobku v souladu s příslušnou harmonizovanou technickou specifikací podle předpokladu výrobce:

Obecný typ	Vlepovaná chemická kotva s kontrolovatelným utahovacím momentem
Použití	Trhlinový nebo netrhlinový beton C20/25 - C50/60 (EN 206)
Úroveň / kategorie	1
Zatížení	Statické nebo kvazi-statické, zemětřesení kategorie (Velikosti VMZ: M10, M12, M16, M20, M24)
Materiál	<p><u>Galvanicky pozinkovaná ocel nebo Diffusion pozinkované:</u> Pouze pro suché vnitřní prostředí Rozměrová řada: VMZ: M8, M10, M12, M16, M20, M24</p> <p><u>Diffusion pozinkované ocel :</u> Pouze pro suché vnitřní prostředí Rozměrová řada: VMZ-IG: M6, M8, M10, M12, M16, M20</p> <p><u>Pozinkovaná ocel:</u> Pouze pro suché vnitřní prostředí Rozměrová řada: VMZ: M8, M10, M12, M16, M20, M24 VMZ-IG: M6, M8, M10, M12, M16, M20</p> <p><u>Nerezová ocel (značení A4):</u> Pro vnitřní i venkovní použití mimo specifické agresivní podmínky Rozměrová řada: VMZ: M8, M10, M12, M16, M20, M24 VMZ-IG: M6, M8, M10, M12, M16, M20</p> <p><u>Vysoce odolná ocel vůči korozí (označení HCR):</u> Pro vnitřní i venkovní použití mimo specifické agresivní podmínky Rozměrová řada: VMZ: M8, M10, M12, M16, M20, M24 VMZ-IG: M6, M8, M10, M12, M16, M20</p>
Teplotní rozsah (pokud to je relevantní)	Rozsah I: -40 °C - +80 °C Rozsah II: -40 °C - +120 °C

4. Jméno, firma nebo registrovaná obchodní známka a kontaktní adresa výrobce podle čl. 11 odst. 5:

MKT Metall-Kunststoff-Technik GmbH & Co. KG
Auf dem Immel 2
D - 67685 Weilerbach

5. Případně jméno a kontaktní adresa zplnomocněného zástupce, jehož plná moc se vztahuje na úkoly uvedené v čl. 12 odst. 2: --
6. Systém nebo systémy posuzování a ověřování stálosti vlastností stavebních výrobků, jak je uvedeno v příloze V:
Systém 1

7. V případě prohlášení o vlastnostech týkajících se stavebního výrobku, na který se vztahuje harmonizovaná norma
8. V případě prohlášení o vlastnostech týkajících se stavebního výrobku, pro který bylo vydané evropské technické posouzení:

Deutsches Institut für Bautechnik, Berlin

vydal:

ETA-04/0092

na základě

ETAG 001-5

Oznámený subjekt 1343-CPR provedl podle systému 1:

- i) určení typu výrobku na základě zkoušky typu (včetně odběru vzorků), výpočtu pro typ, tabulkových hodnot nebo popisné dokumentace výrobku
- ii) počáteční inspekce ve výrobním závodě a řízení výroby
- iii) průběžného dozoru, posouzení a hodnocení řízení výroby

Na základě: osvědčení o stálosti vlastnosti 1343-CPR-M 550-4/08.14

9. Deklarované vlastnosti:

Základní charakteristiky	Návrhová metoda	Provedení		Harmonizovaná technická specifikace
		VMZ-A	VMZ-IG	
Charakteristická únosnost pro tahové napětí	ETAG 001, Annex C CEN/TS 1992-4	Annex C1-C3	Annex C10, C11	ETAG 001
Charakteristická únosnost pro smykové napětí	ETAG 001, Annex C CEN/TS 1992-4	Annex C4, C5	Annex C12	
Charakteristická únosnost pro seismické zatížení	TR 045	Annex C6, C7	--	
Posun kotvy při mezních stavech	ETAG 001, Annex C CEN/TS 1992-4	Annex C8, C9	Annex C12	

Pokud byla použita podle článku 37 nebo 38 specifická technická dokumentace, požadavky, které výrobek splňuje: --

10. Vlastnost výrobku uvedená v bodě 1 a 2 je ve shodě s vlastností uvedenou v bodě 9.

Toto prohlášení o vlastnostech se vydává na výhradní odpovědnost výrobce uvedeného v bodě 4.

Podepsáno za výrobce a jeho jménem:



Stefan Weustenhagen
(Managing Director)
Weilerbach, 13.04.2017

i.V. 

Dipl.-Ing. Detlef Bigalke
(Head of Product Development)



Table C1: Characteristic values for tension loads, VMZ-A M8 – M12, cracked concrete, static and quasi-static action

Anchor size VMZ-A		40 M8	50 M8	60 M10	75 M10	75 M12	70 M12	80 M12	95 M12	100 M12	110 M12	125 M12
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]										1,0
Steel failure												
Characteristic tension resistance $N_{Rk,s}$	Steel, zinc plated A4, HCR	[kN]	15 15	18 18	25 25	35 35	49 49	54 54				57 57
Partial safety factor	γ_{Ms}	[-]										1,5
Pull-out												
Characteristic resistance $N_{Rk,p}$ in concrete C20/25	$50^\circ\text{C} / 80^\circ\text{C}^2)$ $72^\circ\text{C} / 120^\circ\text{C}^2)$	[kN]										1) ¹⁾
		[kN]	5	7,5	12	12	12	16	20	20	30	30
Increasing factor	ψ_c	[-]										$\left(\frac{f_{ck,cube}}{25}\right)^{0,5}$
Concrete cone failure												
Effective anchorage depth	$h_{ef} \geq$	[mm]	40	50	60	75	75	70	80	95	100	110
Factor acc. to CEN/TS 1992-4	k_{cr}	[-]										125

¹⁾ Pull-out failure is not decisive

²⁾ Maximum long term temperature / Maximum short term temperature

Table C2: Characteristic values for tension loads, VMZ-A M16 – M24, cracked concrete, static and quasi-static action

Anchor size VMZ-A		90 M16	105 M16	125 M16	145 M16	160 M16	115 M20	170 M20 (LG)	190 M20 (LG)	170 M24 (LG)	200 M24 (LG)	225 M24 (LG)
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]										1,0
Steel failure												
Characteristic tension resistance $N_{Rk,s}$	Steel, zinc plated A4, HCR	[kN]	88 88	95 95	111 111	97 97	96 114	188 165				222 194
Partial safety factor	γ_{Ms}	[-]				1,5		1,68		1,5		1,5
Pull-out												
Characteristic resistance $N_{Rk,p}$ in concrete C20/25	$50^\circ\text{C} / 80^\circ\text{C}^2)$ $72^\circ\text{C} / 120^\circ\text{C}^2)$	[kN]										1) ¹⁾
		[kN]	25	30	50	51	30	60				75
Increasing factor	ψ_c	[-]										$\left(\frac{f_{ck,cube}}{25}\right)^{0,5}$
Concrete cone failure												
Effective anchorage depth	$h_{ef} \geq$	[mm]	90	105	125	145	160	115	170	190	170	200
Factor acc. to CEN/TS 1992-4	k_{cr}	[-]										225

¹⁾ Pull-out failure is not decisive

²⁾ Maximum long term temperature / Maximum short term temperature

Injection System VMZ

Performance

Characteristic values for tension loads, VMZ-A in cracked concrete, static and quasi-static action

Annex C1

**Table C3: Characteristic values for tension loads, VMZ-A M8 – M12
in uncracked concrete, static and quasi-static action**

Anchor size VMZ-A		40 M8	50 M8	60 M10	75 M10	75 M12	70 M12	80 M12	95 M12	100 M12	110 M12	125 M12	
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]										1,0	
Steel failure													
Characteristic tension resistance $N_{Rk,s}$													
Steel, zinc plated	[kN]	15	18		25		35	49		54		57	
A4, HCR	[kN]	15	18		25		35	49		54		57	
Partial safety factor	γ_{Ms}	[-]										1,5	
Pull-out													
Characteristic resistance $N_{Rk,p}$ in uncracked concrete C20/25		50°C / 80°C ²⁾	[kN]	9	1) ¹⁾		1) ¹⁾		1) ¹⁾	40	1) ¹⁾	50	50
		72°C / 120°C ²⁾	[kN]	6	9		16		16	16	25	25	30
Splitting													
Splitting for standard thickness of concrete member (The higher resistance of Case 1 and Case 2 may be applied.)													
Standard thickness of concrete	$h_{std} \geq 2 h_{ref}$	[mm]	100	120	150	150	140	160	190	200	220	250	
Case 1 ($N_{Rk,c}^0$ has to be replaced by $N_{Rk,sp}^0$)													
Characteristic resistance in uncracked concrete C20/25	$N_{Rk,sp}^0$	[kN]	7,5	9	16	20	20	20	1) ¹⁾	30	40	40	40
Spacing (edge distance)	$s_{cr,sp}$ (= 2 $c_{cr,sp}$)	[mm]							3 h_{ref}				
Case 2													
Spacing (edge distance)	$s_{cr,sp}$ (= 2 $c_{cr,sp}$)	[mm]	6 h_{ref}	5 h_{ref}	7 h_{ref}	7 h_{ref}	5 h_{ref}	3 h_{ref}	5 h_{ref}	4 h_{ref}	6 h_{ref}	5 h_{ref}	
Splitting for minimum thickness of concrete member (The higher resistance of Case 1 and Case 2 may be applied.)													
Minimum thickness of concrete	$h_{min} \geq$	[mm]	80		100	110	110	110	125	130	140	160	
Case 1 ($N_{Rk,c}^0$ has to be replaced by $N_{Rk,sp}^0$)													
Characteristic resistance in uncracked concrete C20/25	$N_{Rk,sp}^0$	[kN]	7,5	-		16		16	20	25	25	30	30
Spacing (edge distance)	$s_{cr,sp}$ (= 2 $c_{cr,sp}$)	[mm]	3 h_{ref}	-		3 h_{ref}					3 h_{ref}		
Case 2													
Spacing (edge distance)	$s_{cr,sp}$ (= 2 $c_{cr,sp}$)	[mm]	6 h_{ref}	7 h_{ref}	6 h_{ref}	7 h_{ref}	7 h_{ref}	7 h_{ref}	6 h_{ref}	7 h_{ref}	6 h_{ref}	6 h_{ref}	
Increasing factor for $N_{Rk,p}$ and $N_{Rk,sp}^0$	ψ_c	[-]							$\left(\frac{f_{ck,cube}}{25}\right)^{0,5}$				
Concrete cone failure													
Effective anchorage depth	$h_{ref} \geq$	[mm]	40	50	60	75	75	70	80	95	100	110	125
Factor acc. to CEN/TS 1992-4	k_{ucr}	[-]								10,1			

¹⁾ Pull-out failure is not decisive

²⁾ Maximum long term temperature / Maximum short term temperature

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Performance

Characteristic values for **tension loads, VMZ-A M8 – M12, uncracked concrete**, static and quasi-static action

Annex C2

Table C4: Characteristic values for tension loads, VMZ-A M16 – M24, uncracked concrete, static and quasi-static action

Anchor size VMZ-A	90 M16	105 M16	125 M16	145 M16	160 M16	115 M20	170 M20 (LG)	190 M20 (LG)	170 M24 (LG)	200 M24 (LG)	225 M24 (LG)
Installation safety factor $\gamma_2 = \gamma_{inst}$	[-]						1,0				
Steel failure											
Characteristic tension resistance $N_{Rk,s}$	Steel, zinc plated A4, HCR	[kN]	88	95	111	111	97	96	188	188	222
Partial safety factor γ_{Ms}	[-]				1,5		1,68		1,5		1,5
Pull-out											
Characteristic resistance $N_{Rk,p}$ in uncracked concrete C20/25	50°C / 80°C ²⁾	[kN]		1)		75	90	1)		1)	
	72°C / 120°C ²⁾	[kN]	25	35	50	50	53	40	75	75	95
Splitting											
Splitting for standard thickness of concrete (The higher resistance of Case 1 and Case 2 may be applied.)											
Standard thickness of concrete	$h_{std} \geq 2 h_{ref}$	[mm]	180	200	250	290	320	230	340	380	340
Case 1 ($N^0_{Rk,c}$ has to be replaced by $N^0_{Rk,sp}$)										400	450
Characteristic resistance in uncracked concrete C20/25	$N^0_{Rk,sp}$	[kN]	40	50	50	60	80	1)	115	1)	140
Spacing (edge distance)	$s_{cr,sp} (= 2 c_{cr,sp})$	[mm]						3 h_{ref}			
Case 2											
Spacing (edge distance)	$s_{cr,sp} (= 2 c_{cr,sp})$	[mm]	4 h_{ref}	3 h_{ref}	3 h_{ref}	4 h_{ref}	3 h_{ref}				
Splitting for minimum thickness of concrete (The higher resistance of Case 1 and Case 2 may be applied.)											
Minimum thickness of concrete	$h_{min} \geq$	[mm]	130	150	160	180	200	160	220	240	220
Case 1 ($N^0_{Rk,c}$ has to be replaced by $N^0_{Rk,sp}$)										260	290
Characteristic resistance in uncracked concrete C20/25	$N^0_{Rk,sp}$	[kN]	35	50	40	50	71	-	75	75	1)
Spacing (edge distance)	$s_{cr,sp} (= 2 c_{cr,sp})$	[mm]						3 h_{ref}			
Case 2											
Spacing (edge distance)	$s_{cr,sp} (= 2 c_{cr,sp})$	[mm]	5 h_{ref}	5 h_{ref}	6 h_{ref}	5 h_{ref}	5 h_{ref}	5 h_{ref}	5,2 h_{ref}	4,4 h_{ref}	5,2 h_{ref}
Increasing factor for $N_{Rk,p}$ and $N^0_{Rk,sp}$	ψ_c	[-]						$\left(\frac{f_{ck,cube}}{25}\right)^{0,5}$			
Concrete cone failure											
Effective anchorage depth	$h_{ref} \geq$	[mm]	90	105	125	145	160	115	170	190	170
Factor acc. to CEN/TS 1992-4	k_{ver}	[-]						10,1		200	225

¹⁾ Pull-out failure is not decisive.

²⁾ Maximum long term temperature / Maximum short term temperature

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Performance

Characteristic values for **tension loads**, VMZ-A M16 – M24, uncracked concrete, static and quasi-static action.

Annex C3

Table C5: Characteristic values for shear load, VMZ-A M8 – M12, cracked and uncracked concrete, static and quasi-static action

Anchor size VMZ-A		40 M8	50 M8	60 M10	75 M10	75 M12	70 M12	80 M12	95 M12	100 M12	110 M12	125 M12
Installation safety factor $\gamma_2 = \gamma_{\text{inst}}$	[-]											1,0
Steel failure without lever arm												
Characteristic shear resistance $V_{Rk,s}$												
Steel, zinc plated	[kN]			14		21						34
A4, HCR	[kN]			15		23						34
Partial safety factor γ_{Ms}	[-]											1,25
Factor for ductility k_2	[-]											1,0
Steel failure with lever arm												
Characteristic bending moments $M_{Rk,s}^0$												
Steel, zinc plated	[Nm]			30		60						105
A4, HCR	[Nm]			30		60						105
Partial safety factor γ_{Ms}	[-]											1,25
Concrete pry-out failure												
Factor k acc. ETAG 001, Annex C or k_3 acc. CEN/TS 1992-4	$k_{(3)}$	[-]										2
Concrete edge failure												
Effective length of anchor in shear load	l_f	[mm]	40	50	60	75	75	70	80	95	100	110
Diameter of anchor	d_{nom}	[mm]		10		12		12				14

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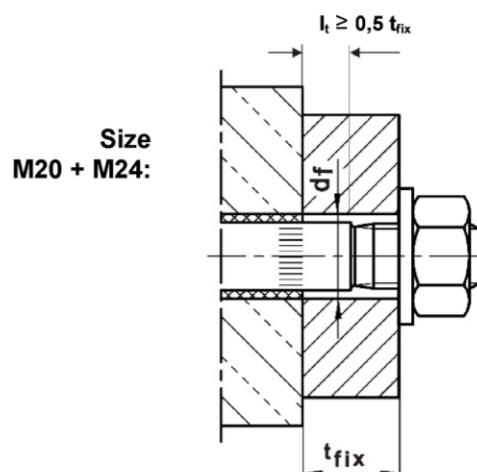
Characteristic values for shear load, VMZ-A M8 – M12, cracked and uncracked concrete, static and quasi-static action

Annex C4

Table C6: Characteristic values for shear load, VMZ-A M16 – M24, cracked and uncracked concrete, static and quasi-static action

Anchor size VMZ-A	90 M16	105 M16	125 M16	145 M16	160 M16	115 M20	170 M20 (LG)	190 M20 (LG)	170 M24 (LG)	200 M24 (LG)	225 M24 (LG)
Installation safety factor $\gamma_2 = \gamma_{\text{inst}}$	[-]										1,0
Steel failure without lever arm											
Characteristic shear resistance $V_{Rk,s}$	Steel, zinc plated A4, HCR	[kN]		63		70	149 ¹⁾ (98)		178 ¹⁾ (141)		
		[kN]		63		86	131 ¹⁾ (86)		156 ¹⁾ (123)		
Partial safety factor γ_{Ms}		[-]		1,25		1,4	1,25		1,25		
Factor for ductility k_2		[-]					1,0				
Steel failure with lever arm											
Characteristic bending moments $M_{Rk,s}^0$	Steel, zinc plated A4, HCR	[Nm]		266		392	519		896		
		[Nm]		266			454		784		
Partial safety factor γ_{Ms}		[-]		1,25		1,4	1,25		1,25		
Concrete pry-out failure											
Factor k acc. ETAG 001, Annex C or k_3 acc. CEN/TS 1992-4	$k_{(3)}$	[-]					2				
Concrete edge failure											
Effective length of anchor in shear load	l_f	[mm]	90	105	125	145	160	115	170	190	170
Diameter of anchor	d_{nom}	[mm]			18			22	24		26

¹⁾ This value may only be applied if $l_t \geq 0,5 t_{\text{fix}}$



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Performance

Characteristic values for shear load, VMZ-A M16 – M24, cracked and uncracked concrete, static and quasi-static action

Annex C5

Table C7: Characteristic resistances for **seismic loading**
VMZ-A M10 – M12 performance category C1 and C2

Anchor size VMZ-A	60 M10	75 M10	75 M12	70 M12	80 M12	95 M12	100 M12	110 M12	125 M12
Tension loads									
Installation safety factor	$\gamma_2 = \gamma_{\text{inst}}$	[–]	1,0						
Steel failure, steel zinc plated									
Characteristic resistance C1	$N_{Rk,s,\text{seis},C1}$	[kN]	25	35	49	54	57		
Characteristic resistance C2	$N_{Rk,s,\text{seis},C2}$	[kN]	25	35	49	54	57		
Steel failure, stainless steel A4, HCR									
Characteristic resistance C1	$N_{Rk,s,\text{seis},C1}$	[kN]	25	35	49	54	57		
Characteristic resistance C2	$N_{Rk,s,\text{seis},C2}$	[kN]	25	35	49	54	57		
Partial safety factor	$\gamma_{Ms,\text{seis}}$	[–]	1,5						
Pull-out									
Characteristic resistance C1	$N_{Rk,p,\text{seis},C1}$	50°C / 80°C ¹⁾	[kN]	14,5	14,5	30,6	36,0	41,5	42,8
		72°C / 120°C ¹⁾	[kN]	10,9	10,9	20,0		30,0	
Characteristic resistance C2	$N_{Rk,p,\text{seis},C2}$	50°C / 80°C ¹⁾	[kN]	7,4	7,4	8,7		17,6	
		72°C / 120°C ¹⁾	[kN]	5,1	5,1	6,5		12,3	

Shear loads									
Steel failure without lever arm, steel zinc plated									
Characteristic resistance C1	$V_{Rk,s,\text{seis},C1}$	[kN]	11,8			27,2			
Characteristic resistance C2	$V_{Rk,s,\text{seis},C2}$	[kN]	12,6			27,2			
Partial safety factor	$\gamma_{Ms,\text{seis}}$	[–]	1,25						
Steel failure without lever arm, stainless steel A4, HCR									
Characteristic resistance C1	$V_{Rk,s,\text{seis},C1}$	[kN]	12,9			27,2			
Characteristic resistance C2	$V_{Rk,s,\text{seis},C2}$	[kN]	13,8			27,2			
Partial safety factor	$\gamma_{Ms,\text{seis}}$	[–]	1,25						
Steel failure with lever arm									
Characteristic bending moment C1	$M_{Rk,s,\text{seis},C1}^0$	[Nm]	no performance determined						
Characteristic bending moment C2	$M_{Rk,s,\text{seis},C2}^0$	[Nm]	no performance determined						

¹⁾ Maximum long term temperature / Maximum short term temperature

Injection System VMZ	Annex C6
Performance Characteristic resistances for seismic loading , VMZ-A M10 – M12, performance category C1 and C2	

Table C8: Characteristic resistances for **seismic loading**
VMZ-A M16 – M24 performance category **C1 and C2**

Anchor size VMZ-A	90 M16	105 M16	125 M16	145 M16	160 M16	115 M20	170 M20 (LG)	190 M20 (LG)	170 M24 (LG)	200 M24 (LG)	225 M24 (LG)
Tension loads											
Installation safety factor $\gamma_2 = \gamma_{\text{inst}}$ [-]											
Steel failure, steel zinc plated											
Characteristic resistance C1 $N_{Rk,s,\text{seis},C1}$ [kN]	88	95	111	97	96	188			222		
Characteristic resistance C2 $N_{Rk,s,\text{seis},C2}$ [kN]	88	95	111	97	96	188			222		
Steel failure, stainless steel A4, HCR											
Characteristic resistance C1 $N_{Rk,s,\text{seis},C1}$ [kN]	88	95	111	97	114	165			194		
Characteristic resistance C2 $N_{Rk,s,\text{seis},C2}$ [kN]	88	95	111	97	114	165			194		
Partial safety factor $\gamma_{Ms,\text{seis}}$ [-]			1,5		1,68	1,5			1,5		
Pull-out											
Characteristic resistance C1 $N_{Rk,p,\text{seis},C1}$	50°C / 80°C ¹⁾ [kN]	30,7	38,7	43,7	44,4	88,2			90,7		
	72°C / 120°C ¹⁾ [kN]	25,0	30,0	38,5	29,4	55,8			59,3		
Characteristic resistance C2 $N_{Rk,p,\text{seis},C2}$	50°C / 80°C ¹⁾ [kN]	16,3	22,1	26,1	30,9	59,7			59,7		
	72°C / 120°C ¹⁾ [kN]	10,5	14,4	19,5	16,2	44,4			44,4		

Shear loads							
Steel failure without lever arm, steel zinc plated							
Characteristic resistance C1 $V_{Rk,s,\text{seis},C1}$ [kN]		39,1		39,1	82,3	107	
Characteristic resistance C2 $V_{Rk,s,\text{seis},C2}$ [kN]		50,4		51,0	108,8 ¹⁾ (71,5)	154,9 ¹⁾ (122,7)	
Partial safety factor $\gamma_{Ms,\text{seis}}$ [-]		1,25		1,4	1,25	1,25	
Steel failure without lever arm, stainless steel A4, HCR							
Characteristic resistance C1 $V_{Rk,s,\text{seis},C1}$ [kN]		39,1		39,1	72,2	93	
Characteristic resistance C2 $V_{Rk,s,\text{seis},C2}$ [kN]		50,4		62,6	95,6 ¹⁾ (62,8)	135,7 ¹⁾ (107)	
Partial safety factor $\gamma_{Ms,\text{seis}}$ [-]		1,25		1,4	1,25	1,25	
Steel failure with lever arm							
Characteristic bending moment C1 $M_{Rk,s,\text{seis},C1}^0$ [Nm]			no performance determined				
Characteristic bending moment C2 $M_{Rk,s,\text{seis},C2}^0$ [Nm]			no performance determined				

¹⁾ This value may only be applied if $l_t \geq 0,5 t_{\text{fix}}$, (see Annex C5)

Injection System VMZ	Annex C7
Performance Characteristic resistances for seismic loading , VMZ-A M16 – M24, performance category C1 and C2	

Table C9: Displacements under tension loads, VMZ-A M8 – M12

Anchor size VMZ-A			40 M8	50 M8	60 M10	75 M10	75 M12	70 M12	80 M12	95 M12	100 M12	110 M12	125 M12
Tension load in cracked concrete	N	[kN]	4,3	6,1	8,0	11,1	11,1	10,0	12,3	15,9	17,1	19,8	24,0
Displacement	δ_{N0}	[mm]	0,5	0,5	0,5	0,6	0,6	0,6	0,6	0,6	0,6	0,7	0,7
	$\delta_{N\infty}$	[mm]											1,3
Tension load in uncracked concrete	N	[kN]	4,3	8,5	11,1	15,6	15,6	14,1	17,2	19,0	24,0	23,8	23,8
Displacement	δ_{N0}	[mm]	0,2	0,4	0,4	0,4	0,4	0,4	0,4	0,4	0,4	0,6	0,6
	$\delta_{N\infty}$	[mm]											1,3
Displacements under seismic tension loads C2													
Displacements for DLS	$\delta_{N,\text{seis,C2(DLS)}}$	[mm]	-	-		1,0		1,0		1,3		1,1	
Displacements for ULS	$\delta_{N,\text{seis,C2(ULS)}}$	[mm]	-	-		3,0		3,0		3,9		3,0	

Table C10: Displacements under tension loads, VMZ-A M16 – M24

Anchor size VMZ-A			90 M16	105 M16	125 M16	145 M16	160 M16	115 M20	170 M20 (LG)	190 M20 (LG)	170 M24 (LG)	200 M24 (LG)	225 M24 (LG)
Tension load in cracked concrete	N	[kN]	14,6	18,4	24,0	30,0	34,7	21,1	38,0	44,9	38,0	48,5	57,9
Displacement	δ_{N0}	[mm]	0,7	0,7	0,7	0,8	1,2	0,7	0,8	0,8	0,8	0,9	0,9
	$\delta_{N\infty}$	[mm]			1,3		1,6	1,1		1,3			1,3
Tension load in uncracked concrete	N	[kN]	20,5	25,9	33,0	35,7	48,1	29,6	53,3	63,0	53,3	67,9	81,1
Displacement	δ_{N0}	[mm]	0,6	0,6	0,6	0,6	0,8	0,5	0,6	0,6	0,6	0,6	0,6
	$\delta_{N\infty}$	[mm]			1,3		1,6	1,1		1,3			1,3
Displacements under seismic tension loads C2													
Displacements for DLS	$\delta_{N,\text{seis,C2(DLS)}}$	[mm]		1,6		1,5		1,7		1,9		1,9	
Displacements for ULS	$\delta_{N,\text{seis,C2(ULS)}}$	[mm]		3,7		4,4		4,0		4,5		4,5	

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Performance
Displacements under tension loads, VMZ-A

Annex C8

Table C11: Displacements under shear loads VMZ-A M8 – M12

Anchor size VMZ-A			40 M8	50 M8	60 M10	75 M10	75 M12	70 M12	80 M12	95 M12	100 M12	110 M12	125 M12
Shear load	V	[kN]	8,3		13,3		19,3						
Displacements	δ_{V0}	[mm]	2,4	2,5	2,9	3,3							
	$\delta_{V\infty}$	[mm]	3,6	3,8	4,4	5,0							
Displacements under seismic shear loads C2													
Displacements for DLS	$\delta_{V,\text{seis},C2(\text{DLS})}$	[mm]	-	-	2,1	2,5							
Displacements for ULS	$\delta_{V,\text{seis},C2(\text{ULS})}$	[mm]	-	-	3,7	5,1							

Table C12: Displacements under shear loads VMZ-A M16 – M24

Anchor size VMZ-A			90 M16	105 M16	125 M16	145 M16	160 M16	115 M20	170 M20 (LG)	190 M20 (LG)	170 M24 (LG)	200 M24 (LG)	225 M24 (LG)		
Shear load	V	[kN]	36			44	75 (49)		89 (71)						
Displacements	δ_{V0}	[mm]	3,8			3,0	4,3 (3,0)		4,6 (3,5)						
	$\delta_{V\infty}$	[mm]	5,7			4,5	6,5 (4,5)		6,9 (5,3)						
Displacements under seismic shear loads C2															
Displacements for DLS	$\delta_{V,\text{seis},C2(\text{DLS})}$	[mm]	2,9			3,5			3,7						
Displacements for ULS	$\delta_{V,\text{seis},C2(\text{ULS})}$	[mm]	6,8			9,3			9,3						

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Performance

Displacements under shear loads, VMZ-A

Annex C9

Table C13: Characteristic values for tension load, VMZ-IG , cracked concrete

Anchor size VMZ-IG		40 M6	50 M6	60 M8	75 M8	70 M10	80 M10	90 M12	105 M12	125 M12	115 M16	170 M16	170 M20
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]											1,0
Steel failure													
Characteristic tension resistance $N_{Rk,s}$													
Steel, zinc plated	[kN]	15	16	19	29	35		67		52	125	108	
A4, HCR	[kN]	11		19	21	33		47		65	88	94	
Partial safety factor	γ_{Ms}	[-]						1,5					
Pull-out													
Characteristic resistance $N_{Rk,p}$ in cracked concrete C20/25	50°C / 80°C ²⁾	[kN]											1)
	72°C / 120°C ²⁾	[kN]	5	7,5	12	16	20	20	30	50	30	60	75
Increasing factor	ψ_c	[-]						$\left(\frac{f_{ck,cube}}{25}\right)^{0,5}$					
Concrete cone failure													
Effective anchorage depth	h_{ef}	[mm]	40	50	60	75	70	80	90	105	125	115	170
Factor according to CEN/TS 1992-4	k_{cr}	[-]							7,2				

¹⁾ Pull-out failure is not decisive

²⁾ Maximum long term temperature / Maximum short term temperature

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Performance

Characteristic values for tension load, VMZ-IG, cracked concrete

Annex C10

Table C14: Characteristic values for tension load, VMZ-IG, uncracked concrete

Anchor size VMZ-IG		40 M6	50 M6	60 M8	75 M8	70 M10	80 M10	90 M12	105 M12	125 M12	115 M16	170 M16	170 M20
Installation safety factor	$\gamma_2 = \gamma_{\text{inst}}$	[-]											1,0
Steel failure													
Characteristic tension resistance $N_{Rk,s}$													
Steel, zinc plated	[kN]	15	16	19	29	35		67		52	125	108	
A4, HCR	[kN]		11	19	21	33		47		65	88	94	
Partial safety factor	γ_{Ms}	[-]						1,5					
Pull-out													
Characteristic resistance $N_{Rk,p}$ in uncracked concrete C20/25													
50°C / 80°C ²⁾	[kN]	9	1)										
72°C / 120°C ²⁾	[kN]	6	9	16	16	25	25	35	50	40	75	95	
Splitting													
Splitting for standard thickness of concrete (The higher resistance of Case 1 and Case 2 may be applied.)													
Standard thickness of concrete	$h_{\text{std}} \geq 2h_{\text{ef}}$	[mm]	100	120	150	140	160	180	200	250	230	340	340
Case 1 ($N_{Rk,c}^0$ has to be replaced by $N_{Rk,sp}^0$)													
Characteristic resistance in concrete C20/25	$N_{Rk,sp}^0$	[kN]	7,5	9	16	20	20	1)	40	50	50	1)	1)
Spacing (edge distance)	$s_{\text{cr},sp}$ (= 2 $c_{\text{cr},sp}$)	[mm]						3 h_{ef}					
Case 2													
Spacing (edge distance)	$s_{\text{cr},sp}$ (= 2 $c_{\text{cr},sp}$)	[mm]	6 h_{ef}	6 h_{ef}	5 h_{ef}	7 h_{ef}	5 h_{ef}	3 h_{ef}	4 h_{ef}	4 h_{ef}	4 h_{ef}	3 h_{ef}	3 h_{ef}
Splitting for minimum thickness of concrete (The higher resistance of Case 1 and Case 2 may be applied.)													
Minimum thickness of concrete	$h_{\text{min}} \geq$	[mm]	80	100	110	110		130	150	160	160	220	220
Case 1 ($N_{Rk,c}^0$ has to be replaced by $N_{Rk,sp}^0$)													
Characteristic resistance in concrete C20/25	$N_{Rk,sp}^0$	[kN]	7,5	-	16	20	25	35	50	40	-	75	1)
Spacing (edge distance)	$s_{\text{cr},sp}$ (= 2 $c_{\text{cr},sp}$)	[mm]						3 h_{ef}					
Case 2													
Spacing (edge distance)	$s_{\text{cr},sp}$ (= 2 $c_{\text{cr},sp}$)	[mm]	6 h_{ef}	7 h_{ef}	6 h_{ef}	7 h_{ef}	7 h_{ef}	6 h_{ef}	5 h_{ef}	5 h_{ef}	6 h_{ef}	5 h_{ef}	5,2 h_{ef}
Increasing factor for $N_{Rk,p}$ and $N_{Rk,sp}$	Ψ_c	[-]						$\left(\frac{f_{ck,\text{cube}}}{25}\right)^{0,5}$					
Concrete cone failure													
Effective anchorage depth	h_{ef}	[mm]	40	50	60	75	70	80	90	105	125	115	170
Factor according to CEN/TS 1992-4	K_{ucr}	[-]						10,1					

¹⁾ Pull-out failure is not decisive

²⁾ Maximum long term temperature / Maximum short term temperature

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Performance

Characteristic values for tension loads, VMZ-IG, uncracked concrete

Annex C11

Table C15: Characteristic values for shear load, VMZ-IG, cracked and uncracked concrete

Anchor size VMZ-IG		40 M6	50 M6	60 M8	75 M8	70 M10	80 M10	90 M12	105 M12	125 M12	115 M16	170 M16	170 M20
Installation safety factor $\gamma_2 = \gamma_{\text{inst}}$		[-]	1,0										
Steel failure without lever arm													
Characteristic shear resistance $V_{Rk,s}$	Steel, zinc plated [kN]	8,0	9,5	15	18		34		26	63	54		
	A4, HCR [kN]	5,5	9,5	10	16		24		32	44	47		
Partial safety factor γ_{Ms}	[-]	1,25											
Factor for ductility k_2	[-]	1,0											
Steel failure with lever arm													
Characteristic bending moments $M_{Rk,s}^0$	Steel, zinc plated [kN]	12	30	60		105		212	266	519			
	A4, HCR [kN]	8,5	21	42		74		187	187	365			
Partial safety factor γ_{Ms}	[-]	1,25											
Concrete pry-out failure													
Factor k acc. ETAG 001, Annex C or k_3 acc. CEN/TS 1992-4	$k_{(3)}$	[-]	2										
Concrete edge failure													
Effective length of anchor in shear load	l_f [mm]	40	50	60	75	70	80	90	105	125	115	170	170
Diameter of anchor	d_{nom} [mm]	10		12		14		18		22	24	26	

Table C16: Displacements under tension loads, VMZ-IG

Anchor size VMZ-IG		40 M6	50 M6	60 M8	75 M8	70 M10	80 M10	90 M12	105 M12	125 M12	115 M16	170 M16	170 M20
Tension load in cracked concrete	N [kN]	4,3	6,1	8,0	11,1	10,0	12,3	14,6	18,4	24,0	21,1	38,0	38,0
Displacement	δ_{N0} [mm]	0,5	0,5	0,6	0,6		0,7				0,7	0,8	0,8
	$\delta_{N\infty}$ [mm]	1,3											1,1
Tension load in uncracked concrete	N [kN]	4,3	8,5	11,1	15,6	14,1	17,2	20,5	25,9	33,0	29,6	53,3	53,3
Displacement	δ_{N0} [mm]	0,2	0,4	0,4		0,4		0,6			0,5	0,6	0,6
	$\delta_{N\infty}$ [mm]	1,3											1,1

Table C17: Displacements under shear loads, VMZ-IG

Anchor size VMZ-IG		40 M6	50 M6	60 M8	75 M8	70 M10	80 M10	90 M12	105 M12	125 M12	115 M16	170 M16	170 M20
Shear load Steel, zinc plated	V [kN]	4,6		5,4	8,4	10,1		19,3			14,8	35,8	30,7
Displacement	δ_{V0} [mm]	0,4		0,5	0,4	0,5		1,2			0,8	1,9	1,2
	$\delta_{V\infty}$ [mm]	0,7		0,8	0,7	0,8		1,9			1,2	2,8	1,9
Shear load Stainless steel A4 / HCR	V [kN]	3,2		5,4	5,9	9,3		13,5			18,5	25,2	26,9
Displacement	δ_{V0} [mm]	0,3		0,5	0,3	0,5		0,9			1,0	1,4	1,1
	$\delta_{V\infty}$ [mm]	0,4		0,7	0,5	0,7		1,4			1,5	2,1	1,6

Injection System VMZ

Performance

Characteristic values for shear load, VMZ-IG, cracked and uncracked concrete, Displacements

Annex C12